

OAPC – CPATT Study: Performance Testing Update

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- Bachelors of Engineering Materials Engineering
 - McMaster University
- Began career in a Mix Design Lab in 2006
- Master of Applied Science Civil Engineering (2011)
- Managed QC, QMS, and Corporate Research & Mix Design Lab for Miller Paving Limited (6 years)
- Currently Canadian Regional Engineer for the Asphalt Institute, since January 2018

Background



Asphalt binders specified based on their properties in an original state

- Performance Graded by AASHTO or ASTM
- What about properties of in-situ asphalt mixtures?
 - Research
 - Forensic investigation
 - Evaluate properties of blended asphalt binder with RAP
 - Predict performance





Agencies are looking for ways to evaluate the properties of the blended asphalt binder (i.e., new binder and old binder from RAP).

Options:

- 1. Solvent extraction-recovery testing on the asphalt mixture
- 2. Asphalt mix performance testing

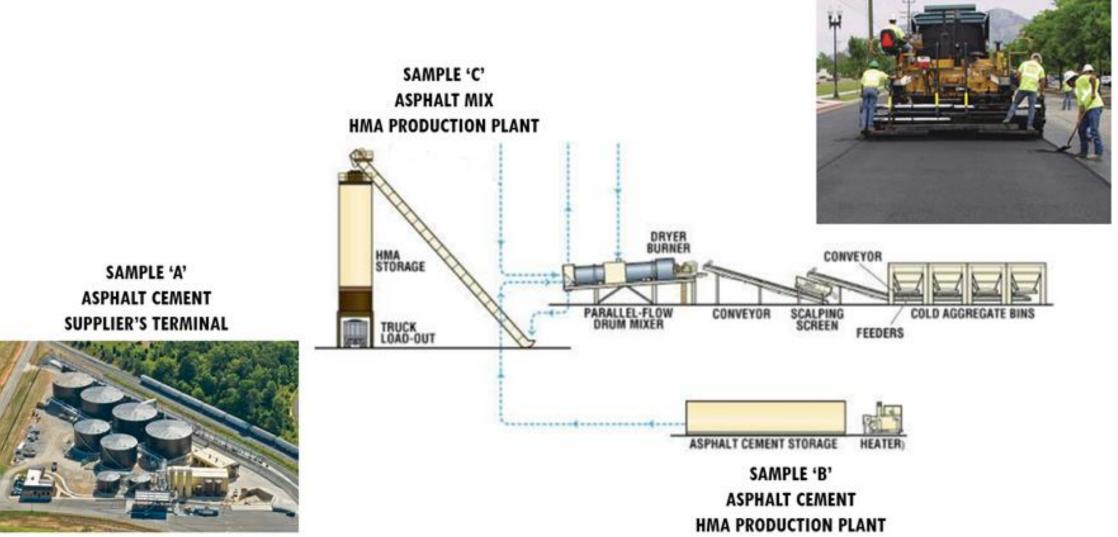


- 1. Evaluate the inter-laboratory standard deviation of the test methods utilized for acceptance of asphalt binders in Ontario.
- 2. Evaluate the asphalt binders and mixes using recent test methods that have been verified through field performance.

Methodology - Sampling



SAMPLE 'D' ASPHALT MIX PAVING SITE





ID	Asphalt Mix Class	PG Grade	RAP Content
1-0708	12.5FC2	70-28	0
2-0809	12.5FC2	70-28	15
3-0915	12.5	58-34	15
4-1003	12.5	58-34	0
6-1006	12.5	58-28	0
7-1010	12.5FC2	64-28	0
8-1031	12.5FC1	58-34	0





Test Description	Test Method/Standard
Extraction of Asphalt Cement and Analysis of Extracted Aggregate	MTO LS 282
Recovery of Asphalt from Solution by Abson or Rotary Evaporator	MTO LS 284
Ash Content	MTO LS 227
Grading or Verifying the Performance Grade of an Asphalt Binder	AASHTO R 29
Multiple Stress Creep Recovery Test of Asphalt Binder Using a Dyna	mic AASHTO T 350
Shear Rheometer (DSR)	
Performance Grade of Physically Aged Asphalt Cement using Exten	ded MTO LS 308
Bending Beam Rheometer (ExBBR)	
Asphalt Cement's Resistance to Ductile Failure Using Double Edge Notc	hed MTO LS 299
Tension (DENT) Test	
Accelerated Aging of Asphalt using Pressure Aging Vessel Protocols	MTO LS 228



The t test is used to test the hypothesis that there is no statistically significant difference in the means of the two groups: tank asphalt and recovered asphalt.

The null hypothesis symbolically is: $H_o: \mu_{tank} = \mu_{recovered}$

p value less than 0.05 means there is less than 5% chance a result in the sample occurred by chance, therefore the results are statistically significant, and reject the null hypothesis that there is no difference.



The null hypothesis symbolically is: $H_o: \mu_{tank} = \mu_{recovered}$

				1	р	values			
PGAC Grade	RAP	Mix ID	Ash	PG High	PG Low	MSCR Jnr	Grade Loss	LTLG	СТОД
58-34	0	8-1031	0.02	0.11	0.65	0.45	0.25	0.97	0.01
58-34	0	4-1003	0.01	0.04	0.86	0.72	0.48	0.08	0.00
58-34	15	3-0915	0.00	0.00	0.09	0.94	0.40	0.02	0.00
58-28	0	6-1006	0.00	0.83	3 0.41 0.2		0.28	0.28	0.24
64-28	0	7-1010	0.01	0.23	0.03	0.59	0.57	0.02	0.06
70-28	0	1-0708	0.00	0.03	0.26	0.40	0.58	0.26	0.20
70-28	15	2-0809	0.00	0.04	0.11	0.76	0.89	0.02	0.04



The significance level is 5 percent (p value 0.05)

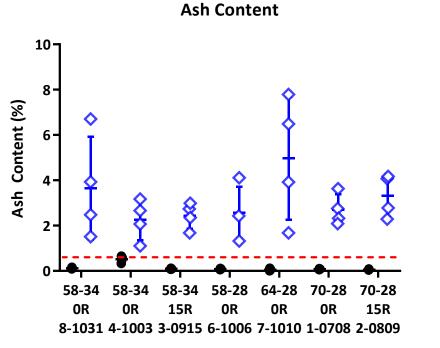
			p values											
PGAC Grade	RAP	Mix ID		PG High	PG Low	MSCR Jnr	Grade Loss	LTLG	СТОД					
58-34	0	8-1031	0.02	0.11	0.65	0.45	0.25	0.97	0.01					
58-34	0	4-1003	0.01	0.04	0.86	0.72	0.48	0.08	0.00					
58-34	15	3-0915	0.00	0.00	0.09	0.94	0.40	0.02	0.00					
58-28	0	6-1006	0.00	0.83	0.41	0.21	0.28	0.28	0.24					
64-28	0	7-1010	0.01	0.23	0.03	0.59	0.57	0.02	0.06					
70-28	0	1-0708	0.00	0.03	0.26	0.40	0.58	0.26	0.20					
70-28	15	2-0809	0.00	0.04	0.11	0.76	0.89	0.02	0.04					

Results - ILS

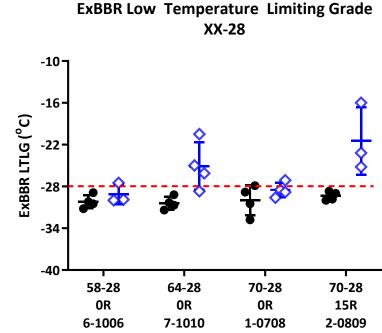


Tank Asphalt

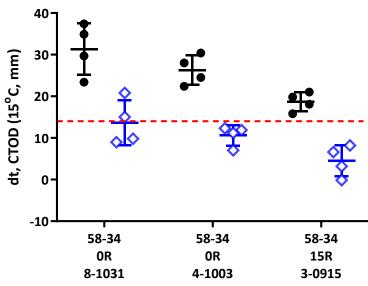
Recovered Asphalt



There is a statistically significant difference in test results for tank and recovered asphalt when testing for ash content.



Critical Tip Opening Displacement XX-34

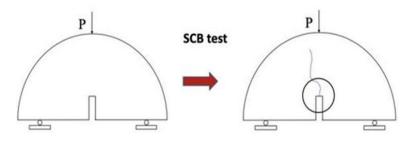


There is a statistically significant difference in test results for LTLG for: PG 64-28, PG 58-28, and PG 70-28 when 15 percent RAP is incorporated.

There is a statistically significant difference in test results for CTOD when RAP is incorporated in the mix and recovered for testing.

- Delta Tc
- Flow Number
- Illinois Flexibility Index Test





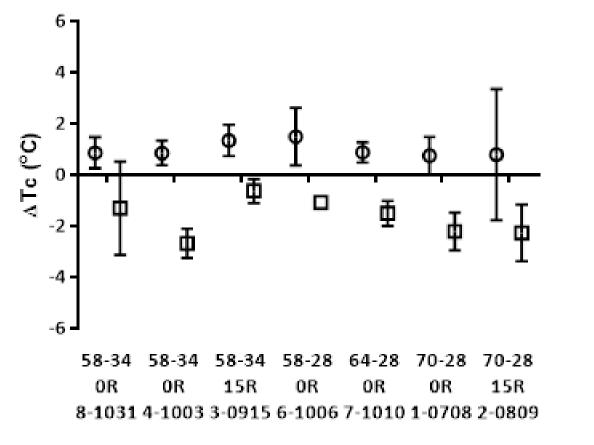






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 ΔT_c is calculated using values from the Bending Beam Rheometer (BBR) test included in the PG system, by subtracting the BBR m-critical temperature from the BBR stiffness-critical temperature: $\Delta T_{c} = (T_{s-critical} - T_{m-critical})$



20Hr PAV

40Hr PAV

 ΔTc becoming worse (more negative) with extended aging, which supports the understanding of the impact of oxidation on the asphalt properties.

% of change in ΔT_c between the 20 and 40 hours is not consistent for all the asphalt binder grades

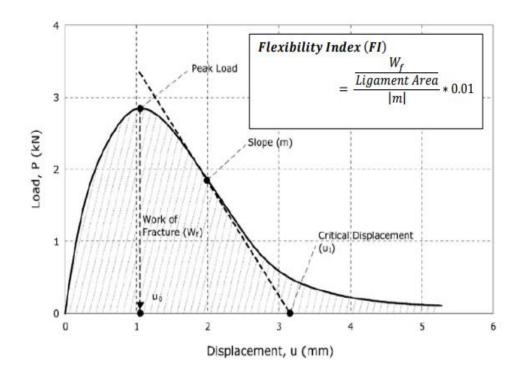


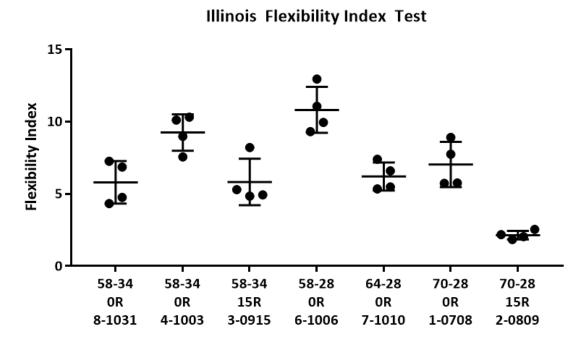
Sample ID	58-34 8-1031	58-34 4-1003	58-34 3-0915*	58-28 6-1006	64-28 7-1010	70-28 1-0708	70-28 2-0809*
Flow Number (FN)	67	28	109	78	136	2659	3119
Min	32	25	78	60	123	1680	440
Max	112	35	162	96	144	4190	4732
StDev	41	6	46	18	11	1342	2336
COV	60	20	43	23	8	51	75

The Flow Number values trend in the manner expected, i.e., higher FN values correspond to mixtures that are more resistant to rutting.



The I-FIT test quantifies the cracking resistance of asphalt mixtures using the Flexibility Index (FI), at intermediate temperature.





PG Grades and RAP Content

Sample ID	58-34 8-1031	58-34 4-1003	58-34 3-0915*	58-28 6-1006	64-28 7-1010	70-28 1-0708	70-28 2-0809*
Average FI	5.8	9.2	5.8	10.8	6.2	7.0	2.2
Min	4.3	7.6	4.9	9.3	5.3	5.7	1.8
Max	7.3	10.3	8.2	12.9	7.4	8.9	2.5
StDev	1.5	1.3	1.6	1.6	1.0	1.6	0.3
COV)V 25		27	15	16	22	14

Results – Correlation Table – Tank Asphalt

TANK ASPHALT	RAP	Ash	PG High	PG Low N	1SCR Jnr	MSCR %Rec 3.2kPa	Loss	Grade Loss (40Hr)	LTLG (20Hr)	LTLG (40Hr)	СТОД	Delta Tc (20Hr)	Delta Tc (40Hr)	Flexibility Index	Flow Number	
RAP							-1 = a p	orfort	noaati	ivo lina	oar cor	relatio	n hotw	oon tu	o vari	ahloc
Ash	-0.3						-1 – u p	erjett	neguti				I DELVV			abies
PG High	0.3	-0.3					0 = no l	linear d	orrela	tion be	etweer	n two v	ariable	25		
PG Low	0.1	-0.9	0.3						•••					.	• •	
MSCR Jnr	-0.3	-0.1	-0.7	0.2			1 = a p e	erject p	DOSITIV	e linea	r corre	Ιάτιοη Ι	betwee	en two	variat	bies
MSCR %Rec																
3.2kPa	0.3	0.1	0.8	-0.1	-1.0											
Grade Loss (20Hr)	0.4	-0.1	0.6	0.2	-0.6	0	7									
Grade Loss	0.4	-0.1	0.0	0.2	-0.0	U	./									
(40Hr)	0.2	-0.2	0.2	0.2	0.0	0	.1 0.4									
LTLG (20Hr)	0.5	-0.9	0.1	0.6	0.1	-0	.1 0.0	0.4			_					
LTLG (40Hr)	0.5	-0.8	0.1	0.5	0.0	-0	.1 0.0	0.5	1.0			_				
CTOD	-0.4	0.7	-0.5	-0.8	0.1	-0	.2 -0.7	-0.3	-0.4	-0.3						
Delta Tc (20Hr)	-0.4	0.4	-1.0	-0.4	0.8	-0	<mark>.8</mark> -0.6	-0.1	-0.2	-0.2	0.5	5				
Delta Tc (40Hr)	0.2	-0.4	-0.3	0.1	0.2	-0	.4 -0.6	0.0	0.7	0.7	0.4	1 0.2				
Flexibility Index	-0.7	0.4	-0.7	-0.2	0.7	-0	.6 -0.3	0.1	-0.5	-0.4	0.2	2 0.8	-0.3			
Flow Number	0.4	-0.3	0.9	0.4	-0.5	0	.6 0.6	0.3	0.2	0.2	0.6	-0.9	-0.3	-0.6		



Results – Correlation Table – Recovered Asphalt



RECOVERED ASPHALT	RAP	Ash	PG High	PG Low	MSCR Jnr	MSCR %Rec 3.2kPa	Grade Loss (20Hr)	Grade Loss (40Hr)	LTLG (20Hr)	LTLG (40Hr)	СТОД	Delta Tc (20Hr)	Delta Tc (40Hr)	Flexibility Index	Flow Number
RAP						_1 -	- a nor	fort no	aativo	linoar	correla	ntion h	otwoor	o two v	ariables
Ash	0.2					-7-	- u perj		guive	meur	corren				unubies
PG High	0.3	0.5				0 =	no line	ear cor	relatio	n betw	een tu	vo vari	ables		
PG Low	0.3	0.4	0.5			1 =	a nerf	ert nog	sitive li	near ca	orrelat	ion het	weent	two vai	riahles
MSCR Jnr	-0.3	-0.2	-0.7	0.0)	-	u perj						WCCIII		Tables
MSCR %Rec 3.2kPa	0.1	-0.2	0.5	-0.2	-0.8										
Grade Loss (20Hr)	0.1	0.7	0.5	0.3	-0.3	0.0									
Grade Loss (40Hr)	-0.3	0.4	0.1	0.0	-0.1	-0.1	0.3								
LTLG (20Hr)	0.4	0.5	0.5	0.8	-0.1	-0.2	0.4	0.2							
LTLG (40Hr)	0.1	0.7	0.5	0.6	-0.2	-0.1	0.6	0.5	0.8						
СТОД	-0.3	-0.6	-0.5	-0.6	0.1	0.2	-0.8	-0.3	-0.6	-0.6					
Delta Tc (20Hr)	-0.5	-0.5	-0.5	-0.8	0.2	0.1	-0.3	-0.1	-0.7	-0.6	0.5	5			
Delta Tc (40Hr)	0.1	-0.7	-0.5	-0.2	0.3	0.0	-0.6	-0.5	-0.4	-0.8	0.6	0.3			
Flexibility Index	-0.7	-0.2	-0.6	-0.4	0.6	-0.5	0.0	0.0	-0.4	-0.2	0.1	. 0.5	0.0		
Flow Number	0.4	0.1	. 0.8	0.4	-0.5	0.6	0.2	-0.1	. 0.3	0.1	-0.2	-0.3	0.0	-0.6	



The significant increase in ash content, coupled with the difference in aging for lab aging versus plant production, produced rheological properties that show the recovered asphalt was stiffer and less representative of the tank asphalt, namely: the recovered asphalt had higher PG high temperatures, higher PG low temperatures, higher LTLG, higher Grade Loss, and lower CTOD results.



The physical properties of recovered binder have shown to be statistically different from the physical property tests on tank asphalt.



The alternate tests included in the study: Delta Tc, AMPT Flow number, and I-FIT Flexibility Index correlated well with other measured parameters with verified field performance.

Thank you. Questions?







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